Appendix 9A Hydrology Study

Magnolia Ranch Residential Project
Initial Study



ALLARD ENGINEERING

civil engineering land surveying land planning

APN No. 152-040-003 Riverside County 13175 Orange Street (Magnolia Ranch)

Preliminary Drainage Report

July 12, 2023

Prepared For:

Woodside 05S, LP 1250 Corona Pointe, Suite 500 Corona, CA 92879 (951) 710-1900

Prepared By:

Allard Engineering 16866 Seville Avenue Fontana, CA 92335 (909) 356-1815

Prepared under the supervision of:

Raymond J. Allard

PROFESS/ON A TO NO. 36052
Exp. 6/30/24

CIVIL CRITTED COF CALIFORNY

Raymond J. Allard, P.E. RCE 36052 Exp. 06-30-24

DISCUSSION

Purpose:

The purpose of this report is to provide hydraulic and hydraulic calculations for the proposed APN# 152-040-003 and provide the design of the onsite storm drain systems for the proposed SFR development. The project site is located in the city of Eastvale, County of Riverside. The site is north of Orange Street, south of Schleisman Road, and existing residential/commercial development to the east/west.

Purpose

The purpose of this report is to demonstrate how the developed runoff for the proposed tract of SFR development retained and mitigated water quality volume, and sizing of onsite storm drain system for 100-yr storm event. The retention/infiltration basin overflow into the proposed onsite storm drain system and drains to the existing City storm drain (48" RCP) in Orange Road.

Criteria

The criteria utilized for hydrologic and Hydraulic analysis is the Riverside County Hydrology Manual and Rational Method Hydrology Analysis utilizing AES software to perform computations. 100-yr storm analysis was performed for the developed & un-developed condition.

Findings

Due the preliminary nature of this report, only the 100-yr storm event for the developed hydrology has been analyzed.

In pre-developed condition, the site consist of a single structure residential development and drain out to the south to the existing City Storm Drain System in Orange Street via proposed lateral.

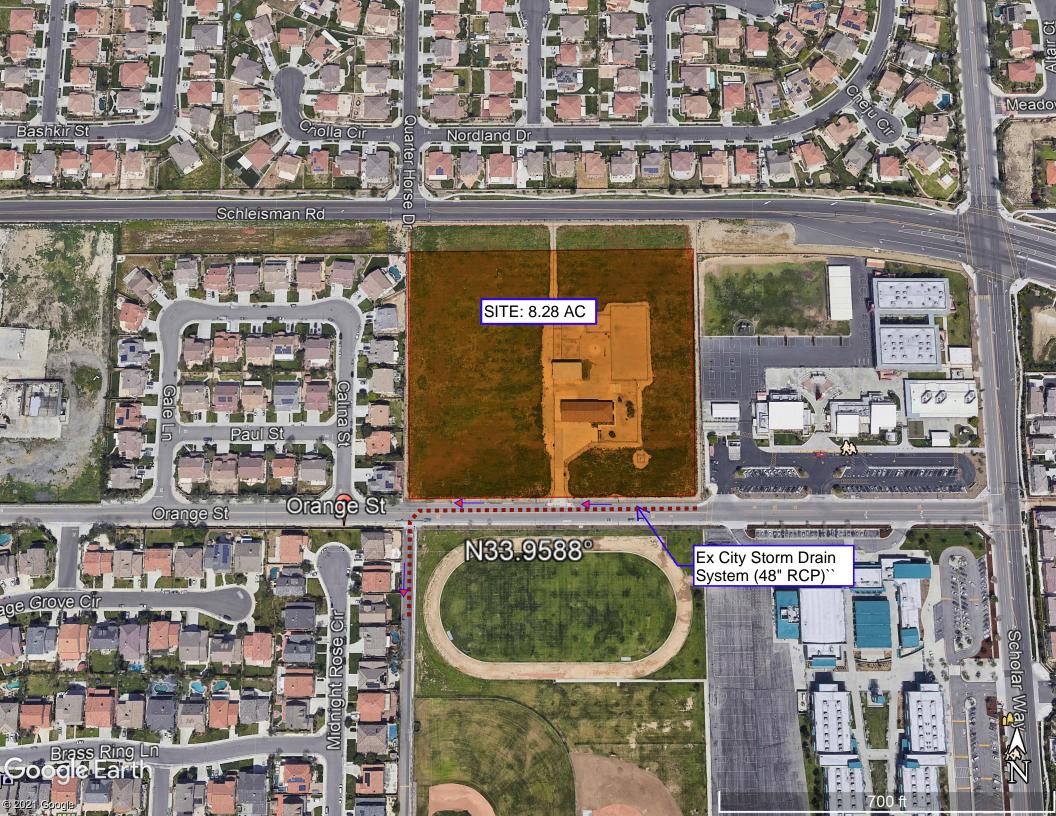
In the proposed developed (Tract of single family residential development) site will drain to the proposed two below surface retention/infiltration chamber system-1,2 for low flow infiltration. Once the retention/infiltration chamber system-1,2 reach its capacity, it will overflow into the onsite storm drain system and finally drains to the existing City Storm Drain System in Orange Street via the proposed 30" RCP lateral. It was calculated in proposed condition the site will generate a peak runoff flowrate of 22.8 from 100-yr storm event. All the proposed onsite storm drainage system are sized based the flow rate for 100-yr storm event. The proposed onsite drainage system conveys runoff to the existing storm drain system south of the site which is also consistence with the existing drainage pattern.

In pre-developed condition the runoff rate calculated as 15.5 CFS and in developed condition the runoff rate calculated as 22.8 CFS. There will be an increase of 7.3 CFS in developed condition. By implementing the Retention/Infiltration Chamber System-1,2 onsite, and by attenuation of runoff in the Retention/Infiltration Chamber System-1,2, the outflow from the site in developed

condition will substantially reduce and will be at or below the pre-developed condition runoff rate.

There is an existing storm drain system in Orange Street across the sites frontage which begins as a 42" and transitions to a 48" part way through the site. This storm drain had a lateral A-2 which was bulk headed off at the front of our site for this development to tie into. Lateral A-2 was estimated to carry 24.7 CFS, which is greater than the 22.8 CFS the project generates. Therefore, this project is consistent with the anticipated flows of the existing City storm drain system, and will not affect the HGL found on the city reference drawings. These plans are included as reference.

Hydrology Reference Material

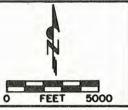




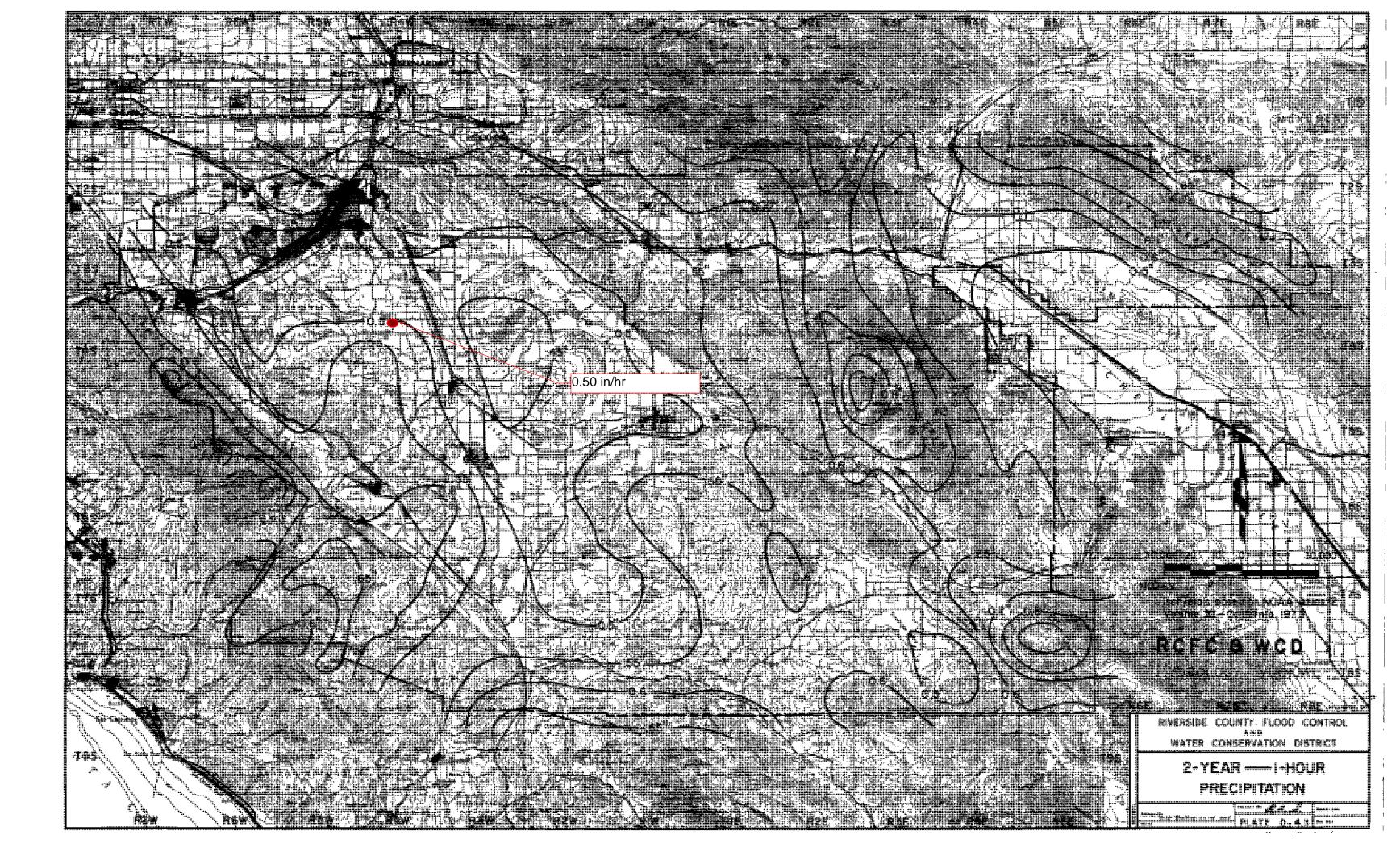
LEGEND SOILS GROUP BOUNDARY SOILS GROUP DESIGNATION

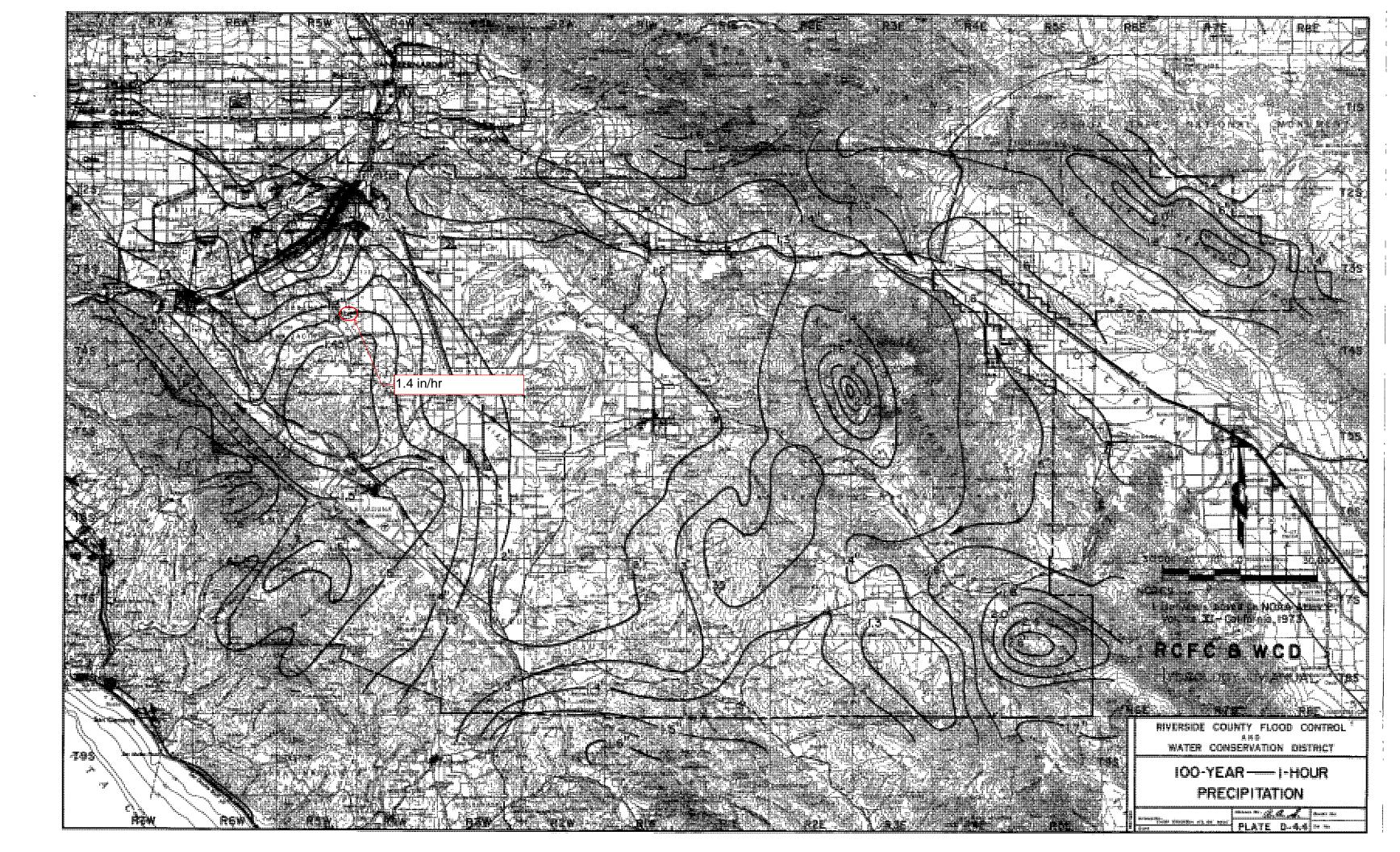
RCFC&WCD

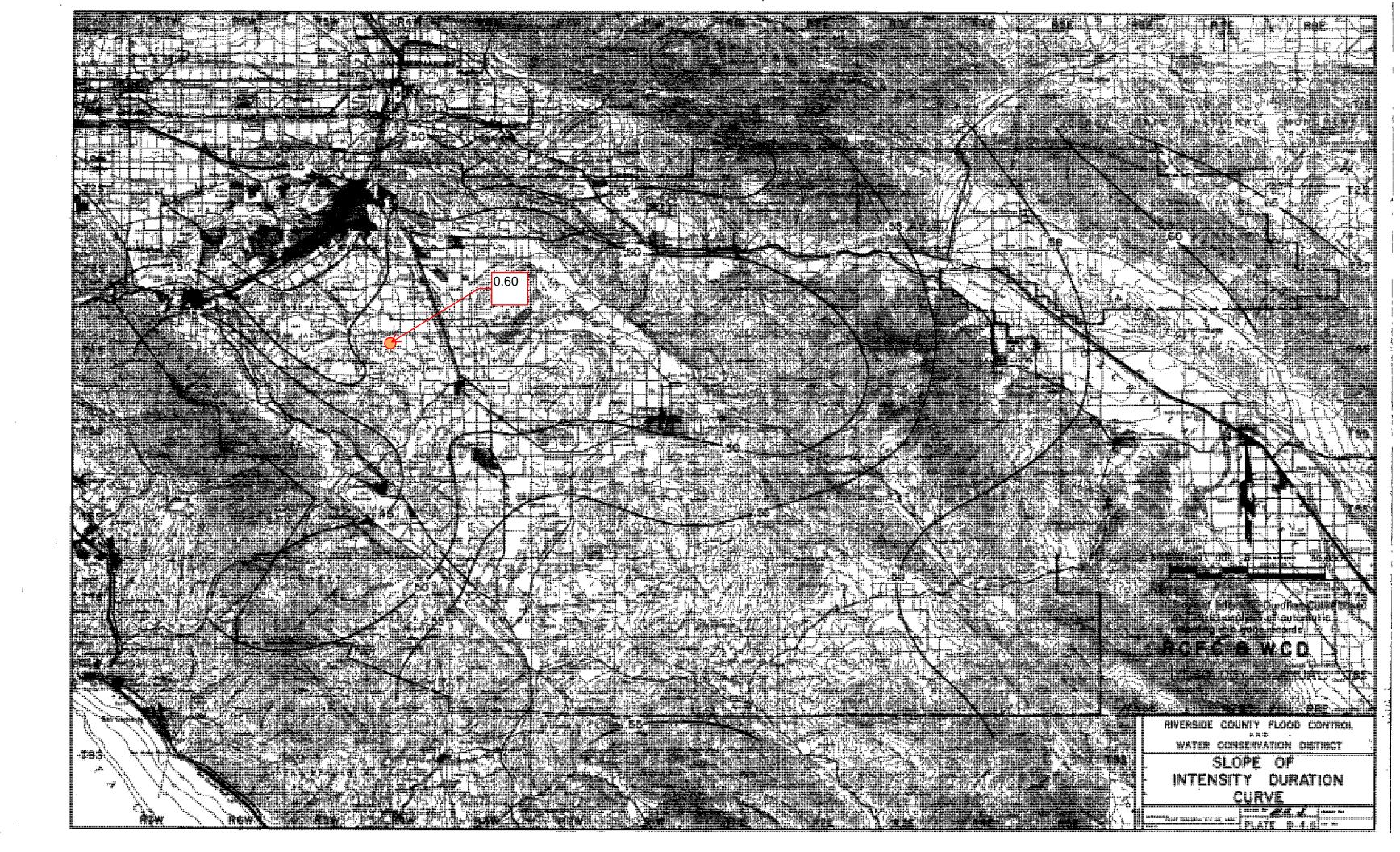
HYDROLOGY MANUAL



HYDROLOGIC SOILS GROUP MAP FOR STEELE PEAK







Rational Method Hydrology Analysis 100-yr Storm Event (Developed Condition)

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM BASED ON RIVERSIDE COUNTY FLOOD CONTROL & WATER CONSERVATION DISTRICT (RCFC&WCD) 1978 HYDROLOGY MANUAL

(c) Copyright 1982-2016 Advanced Engineering Software (aes) (Rational Tabling Version 23.0)

Release Date: 07/01/2016 License ID 1400

Analysis prepared by:

```
* MLC EASTVALE, DEVELOPED CONDITION
* Peak Runoff CAlculation
* 100yr storm event
 FILE NAME: MLC.DAT
 TIME/DATE OF STUDY: 11:02 07/25/2022
 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
 USER SPECIFIED STORM EVENT(YEAR) = 100.00
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 12.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.60
 2-YEAR, 1-HOUR PRECIPITATION(INCH) = 0.550
 100-YEAR, 1-HOUR PRECIPITATION(INCH) = 1.400
 COMPUTED RAINFALL INTENSITY DATA:
 STORM EVENT = 100.00 1-HOUR INTENSITY(INCH/HOUR) =
 SLOPE OF INTENSITY DURATION CURVE = 0.5500
 RCFC&WCD HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: COMPUTE CONFLUENCE VALUES ACCORDING TO RCFC&WCD HYDROLOGY MANUAL
      AND IGNORE OTHER CONFLUENCE COMBINATIONS FOR DOWNSTREAM ANALYSES
 *USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
   HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
   WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
1 30.0 20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150
 GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
   1. Relative Flow-Depth = 0.00 FEET
     as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
   2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
  OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
********************
 FLOW PROCESS FROM NODE 0.00 TO NODE 1.00 IS CODE = 21
 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS
______
       ASSUMED INITIAL SUBAREA UNIFORM
       DEVELOPMENT IS SINGLE FAMILY (1/4 ACRE)
 TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 448.00
 UPSTREAM ELEVATION(FEET) = 618.90
 DOWNSTREAM ELEVATION(FEET) = 613.00
 ELEVATION DIFFERENCE (FEET) =
                            5.90
 TC = 0.393*[(448.00**3)/(5.90)]**.2 = 10.729
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.608
 USER-SPECIFIED RUNOFF COEFFICIENT = .7987
 SOIL CLASSIFICATION IS "B"
 SUBAREA RUNOFF(CFS) = 10.72
TOTAL AREA(ACRES) = 3.72 TOTAL RUNOFF(CFS) = 10.72
```

```
FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 31
______
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) << < <
______
 ELEVATION DATA: UPSTREAM(FEET) = 604.80 DOWNSTREAM(FEET) = 602.50
 FLOW LENGTH(FEET) = 446.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 17.0 INCHES
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.50
 ESTIMATED PIPE DIAMETER(INCH) = 24.00
                             NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 10.72
 PIPE TRAVEL TIME(MIN.) = 1.65 Tc(MIN.) = 12.38
                     0.00 TO NODE
 LONGEST FLOWPATH FROM NODE
                                  2.00 =
                                         894.00 FEET.
**********************
FLOW PROCESS FROM NODE 2.00 TO NODE 2.00 IS CODE = 81
______
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<
______
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.335
 USER-SPECIFIED RUNOFF COEFFICIENT = .7924
 SOIL CLASSIFICATION IS "B"
 SUBAREA AREA(ACRES) = 4.56 SUBAREA RUNOFF(CFS) = 12.05
                  8.3 TOTAL RUNOFF(CFS) =
 TOTAL AREA(ACRES) =
 TC(MIN.) = 12.38
*******************
 FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 31
______
 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) << <<
______
 ELEVATION DATA: UPSTREAM(FEET) = 602.50 DOWNSTREAM(FEET) = 601.90
 FLOW LENGTH(FEET) = 118.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 33.0 INCH PIPE IS 21.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 5.44
 ESTIMATED PIPE DIAMETER(INCH) = 33.00
                             NUMBER OF PIPES =
 PIPE-FLOW(CFS) = 22.77
 PIPE TRAVEL TIME(MIN.) = 0.36 Tc(MIN.) = 12.74
LONGEST FLOWPATH FROM NODE 0.00 TO NODE 3.00 =
                                        1012.00 FEET.
______
 END OF STUDY SUMMARY:
 TOTAL AREA(ACRES)
                     8.3 \text{ TC(MIN.)} =
                                  12.74
 PEAK FLOW RATE(CFS) =
                   22.77
______
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```

END OF RATIONAL METHOD ANALYSIS

Rational Method Hydrology Analysis 100-yr Storm Event (Existing Condition)

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM BASED ON RIVERSIDE COUNTY FLOOD CONTROL & WATER CONSERVATION DISTRICT (RCFC&WCD) 1978 HYDROLOGY MANUAL

(c) Copyright 1982-2016 Advanced Engineering Software (aes) (Rational Tabling Version 23.0)

Release Date: 07/01/2016 License ID 1400

Analysis prepared by:

```
* MLC ONTARION, EXISTING CONDITION
* Peak Runoff CAlculation
* 100yr storm event
********************
 FILE NAME: MLC.DAT
 TIME/DATE OF STUDY: 11:58 07/25/2022
 ______
 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
 ______
 USER SPECIFIED STORM EVENT(YEAR) = 100.00
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 12.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.60
 2-YEAR, 1-HOUR PRECIPITATION(INCH) = 0.550
 100-YEAR, 1-HOUR PRECIPITATION(INCH) = 1.400
 COMPUTED RAINFALL INTENSITY DATA:
 STORM EVENT = 100.00 1-HOUR INTENSITY(INCH/HOUR) =
 SLOPE OF INTENSITY DURATION CURVE = 0.5500
 RCFC&WCD HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: COMPUTE CONFLUENCE VALUES ACCORDING TO RCFC&WCD HYDROLOGY MANUAL
     AND IGNORE OTHER CONFLUENCE COMBINATIONS FOR DOWNSTREAM ANALYSES
 *USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL*
   HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
   WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
         (FT) SIDE / SIDE/ WAY
                               (FT) (FT) (FT) (FT)
NO
   (FT)
1 30.0
          20.0 0.018/0.018/0.020 0.67 2.00 0.0313 0.167 0.0150
 GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
   1. Relative Flow-Depth = 0.00 FEET
     as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
   2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
  OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*****************************
 FLOW PROCESS FROM NODE 0.00 TO NODE 1.00 IS CODE = 21
 _____
 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<
______
      ASSUMED INITIAL SUBAREA UNIFORM
      DEVELOPMENT IS SINGLE FAMILY(1-ACRE LOTS)
 TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
 INITIAL SUBAREA FLOW-LENGTH(FEET) =
 UPSTREAM ELEVATION(FEET) = 618.90
 DOWNSTREAM ELEVATION(FEET) = 613.90
```

```
ELEVATION DIFFERENCE(FEET) = 5.00

TC = 0.469*[( 775.00**3)/( 5.00)]**.2 = 18.418

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.681

USER-SPECIFIED RUNOFF COEFFICIENT = .6977

SOIL CLASSIFICATION IS "B"

SUBAREA RUNOFF(CFS) = 15.49

TOTAL AREA(ACRES) = 8.28 TOTAL RUNOFF(CFS) = 15.49

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 8.3 TC(MIN.) = 18.42

PEAK FLOW RATE(CFS) = 15.49
```

END OF RATIONAL METHOD ANALYSIS

Hydraulic Calculations Developed Condition

12" SD Capacity Calc

TIME/DATE OF STUDY: 16:35 02/25/2021 ______ Problem Descriptions: 12" sd caPACITY ************************ >>>PIPEFLOW HYDRAULIC INPUT INFORMATION<> PIPE DIAMETER(FEET) = 1.000 FLOWDEPTH(FEET) = 0.950PIPE SLOPE(FEET/FEET) = 0.0050 MANNINGS FRICTION FACTOR = 0.013000 >>>> NORMAL DEPTH FLOW(CFS) = ______ NORMAL-DEPTH FLOW INFORMATION: NORMAL DEPTH(FEET) = 0.95 FLOW AREA(SQUARE FEET) = 0.77 FLOW TOP-WIDTH(FEET) = 0.436 FLOW PRESSURE + MOMENTUM(POUNDS) = 40.54 FLOW VELOCITY(FEET/SEC.) = 3.512 FLOW VELOCITY HEAD(FEET) = 0.192 HYDRAULIC DEPTH(FEET) = 1.77 FROUDE NUMBER = 0.465 SPECIFIC ENERGY(FEET) = 1.14

24" SD Capacity Calc

HYDRAULIC ELEMENTS - I PROGRAM PACKAGE

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Ver. 23.0 Release Date: 07/01/2016 License ID 1400

Analysis prepared by:

TIME/DATE OF STUDY: 11:07 07/25/2022 ______ Problem Descriptions: 24" SD CAPACITY CALC S=0.005************************ >>>PIPEFLOW HYDRAULIC INPUT INFORMATION<> PIPE DIAMETER(FEET) = 2.000 FLOWDEPTH(FEET) = 1.900PIPE SLOPE(FEET/FEET) = 0.0050 MANNINGS FRICTION FACTOR = 0.013000 >>>> NORMAL DEPTH FLOW(CFS) = 17.19 ______ NORMAL-DEPTH FLOW INFORMATION: NORMAL DEPTH(FEET) = FLOW AREA(SQUARE FEET) = 3.08 FLOW TOP-WIDTH(FEET) = 0.872 FLOW PRESSURE + MOMENTUM(POUNDS) = 362.61 FLOW VELOCITY(FEET/SEC.) = 5.575 FLOW VELOCITY HEAD(FEET) = 0.483 HYDRAULIC DEPTH(FEET) = 3.54 FROUDE NUMBER = 0.522 SPECIFIC ENERGY(FEET) = 2.38

30" SD Capacity Calc

HYDRAULIC ELEMENTS - I PROGRAM PACKAGE

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Ver. 23.0 Release Date: 07/01/2016 License ID 1400

Analysis prepared by:

TIME/DATE OF STUDY: 10:57 07/25/2022	
Problem Descriptions: 30" SD CAPACITY CALC S=0.005	=======================================
**************************************	*******
PIPE DIAMETER(FEET) = 2.500 FLOWDEPTH(FEET) = 2.450 PIPE SLOPE(FEET/FEET) = 0.0050 MANNINGS FRICTION FACTOR = 0.013000 >>>> NORMAL DEPTH FLOW(CFS) = 30.65	
NORMAL-DEPTH FLOW INFORMATION:	*Q100: 22.8 cfs<30.65 cfs
NORMAL DEPTH(FEET) = 2.45 FLOW AREA(SQUARE FEET) = 4.89 FLOW TOP-WIDTH(FEET) = 0.700	*Developed Condition Peak Runoff Rate
FLOW PRESSURE + MOMENTUM(POUNDS) = FLOW VELOCITY(FEET/SEC.) = 6.273 FLOW VELOCITY HEAD(FEET) = 0.611 HYDRAULIC DEPTH(FEET) = 6.98 FROUDE NUMBER = 0.418 SPECIFIC ENERGY(FEET) = 3.06	740.44 > 22.1 CFS (Developed Condition Peak Runoff Rate)

Street Capacity Calc

HYDRAULIC ELEMENTS - I PROGRAM PACKAGE

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Analysis prepared by:

```
TIME/DATE OF STUDY: 12:31 07/25/2022
______
Problem Descriptions:
 STREET CAPACITY CALCULATION
 DEV. CONDITION
 MLC, EASTVALE
************************
>>>STREETFLOW MODEL INPUT INFORMATION<>
   CONSTANT STREET GRADE(FEET/FEET) = 0.005000
   CONSTANT STREET FLOW(CFS) = 11.00
   AVERAGE STREETFLOW FRICTION FACTOR(MANNING) = 0.015000
   CONSTANT SYMMETRICAL STREET HALF-WIDTH(FEET) = 18.00
   DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) =
   INTERIOR STREET CROSSFALL(DECIMAL) = 0.020000
   OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020000
   CONSTANT SYMMETRICAL CURB HEIGHT(FEET) = 0.50
CONSTANT SYMMETRICAL GUTTER-WIDTH(FEET) = 1.50
   CONSTANT SYMMETRICAL GUTTER-LIP(FEET) = 0.03125
   CONSTANT SYMMETRICAL GUTTER-HIKE(FEET) = 0.12500
   FLOW ASSUMED TO FILL STREET EVENLY ON BOTH SIDES
______
                                      <0.5' OK
   STREET FLOW MODEL RESULTS:
   STREET FLOW DEPTH(FEET) = 0.44
   HALFSTREET FLOOD WIDTH(FEET) = 15.68
   AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.13
   PRODUCT OF DEPTH&VELOCITY = 0.94
```

Catch Basin Sizing Calc

HYDRAULIC ELEMENTS - I PROGRAM PACKAGE
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Ver. 23.0 Release Date: 07/01/2016 License ID 1400

Analysis prepared by:

TIME/DATE OF STUDY: 17:57 03/24/2022

Problem Descriptions:
CATCH BASIN SIZING CALC

>>>FLOWBY CATCH BASIN INLET CAPACITY INPUT INFORMATION

Curb Inlet Capacities are approximated based on the Bureau of Public Roads nomograph plots for flowby basins and sump basins.

STREETFLOW(CFS) = 10.00 GUTTER FLOWDEPTH(FEET) = 0.50 BASIN LOCAL DEPRESSION(FEET) = 0.18

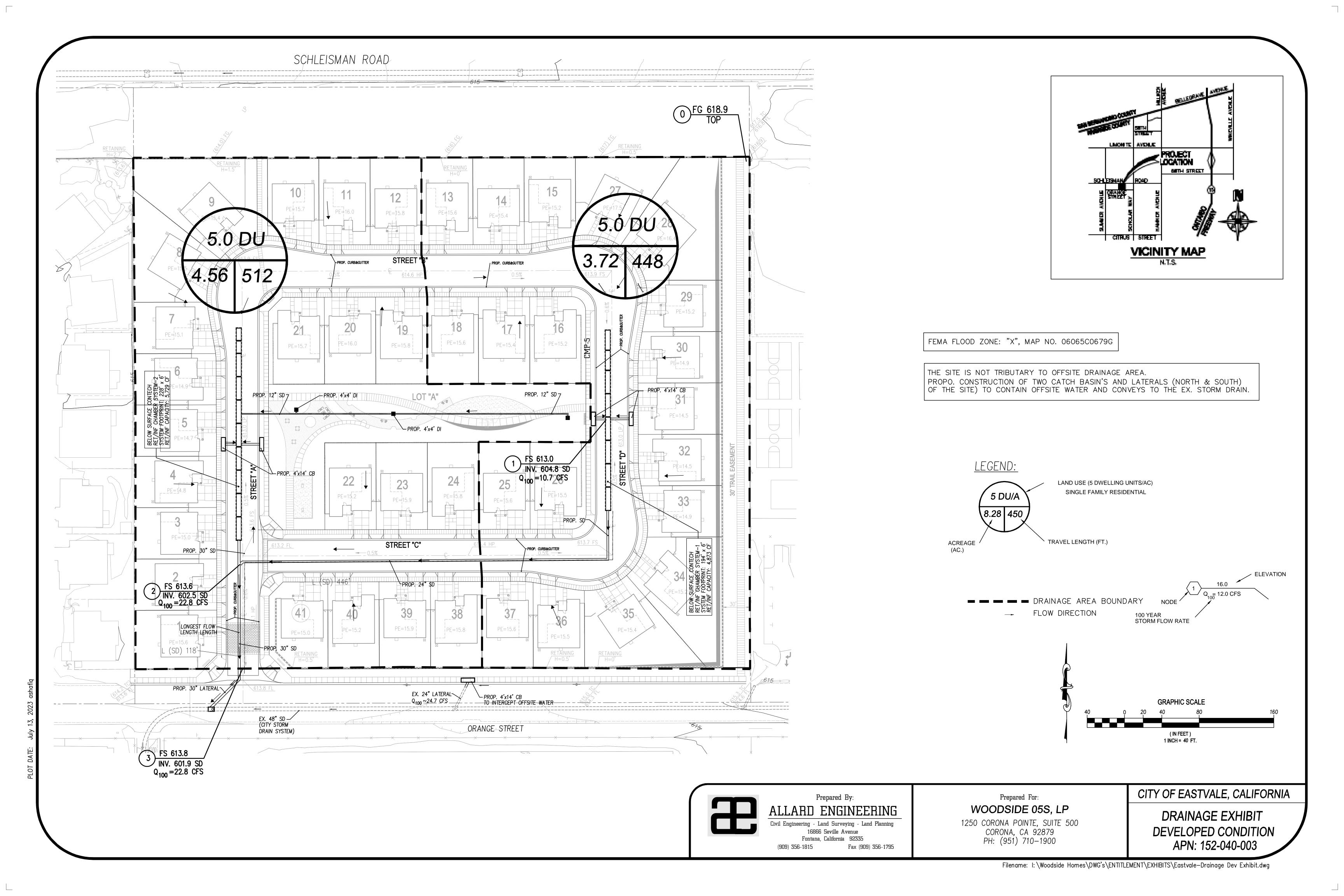
FLOWBY BASIN ANALYSIS RESULTS:

BASIN WIDTH 2.56 3.00 3.50 4.00 4.50 5.00 5.50 6.00 6.50 7.00 7.50 8.00 8.50 9.00 9.50 10.00 10.50 11.00 11.50 12.00 12.50 13.00 13.50 14.00 14.50 15.00 16.50	FLOW INTERCEPTION 1.82 2.11 2.43 2.74 3.05 3.36 3.66 3.96 4.26 4.52 4.78 5.03 5.27 5.51 5.75 5.98 6.21 6.44 6.67 6.89 7.10 7.30 7.49 7.67 7.84 8.00 8.16 8.31 8.45
15.00	8.00
15.50	8.16
16.50	8.45
17.00	8.58
17.50	8.70
18.00	8.82
18.50	8.94
19.00	9.04
19.50	9.15

•

```
20.50 9.33
21.00 9.42
21.50 9.50
22.00 9.57
22.50 9.65
23.00 9.71
23.50 9.78
24.00 9.84
24.50 9.89
25.00 9.94
25.50 9.99
25.57 10.00
```

Drainage Exhibit
Developed Condition
Existing Condition



SCALE: 1"= 100'

Fax (909) 356-1795

Eastvale Master Drainage Plan Lateral D-7 (As-built)

RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT

INDEX

SHEET NO .:

TITLE SHEET

PLAN & PROFILE

2-5

GENERAL NOTES

- THE CONTRACTOR SHALL CONSTRUCT THE FLOOD CONTROL IMPROVEMENTS SHOWN ON THE DRAWINGS IN CONFORMANCE WITH THE REQUIREMENTS OF THE RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT'S M.O.U. STANDARD SPECIFICATIONS DATED SEPTEMBER 1984, AND DESIGN MANUAL STANDARD DRAWINGS DATED MAY 1971.
- AN ENCROACHMENT PERMIT IS REQUIRED FROM RIVERSIDE COUNTY FLOOD CONTROL. CONTACT ED LOTZ AT 909/955-1266. AFTER THE PERMIT IS ISSUED THE DISTRICT MUST BE NOTIFIED ONE WEEK PRIOR TO CONSTRUCTION.
- CONSTRUCTION INSPECTION WILL BE PERFORMED BY RIVERSIDE COUNTY FLOOD CONTROL CONTACT DALE ANDERSON AT 909/955-1288. THE DISTRICT MUST BE NOTIFIED TWENTY DAYS (20) PRIOR TO CONSTRUCTION.
- ALL STATIONING REFERS TO CENTERLINE OF CONSTRUCTION UNLESS OTHERWISE NOTED.
- STATIONING FOR LATERALS AND CONNECTOR PIPE REFER TO THE CENTERLINE INTERSECTION
- FORTY-EIGHT HOURS BEFORE EXCAVATION, CALL UNDERGROUND SERVICE ALERT 1-800-227-2600.
- ALL ELEVATIONS SHOWN ARE IN FEET AND DECIMALS THEREOF BASED ON U.S.C. & G.S DATUM.
- ALL CROSS SECTIONS ARE TAKEN LOOKING DOWNSTREAM.
- ELEVATIONS OF UTILITIES ARE APPROXIMATE UNLESS OTHERWISE NOTED.
- OPENINGS RESULTING FROM THE CUTTING OR PARTIAL REMOVAL OF EXISTING CULVERTS, PIPES OR SIMILAR STRUCTURES TO BE ABANDONED SHALL BE SEALED WITH 6" OF CLASS "B" CONCRETE.
- PIPE CONNECTED TO THE MAINLINE PIPE SHALL CONFORM TO JUNCTION STRUCTURE NO. 4 (JS 229) UNLESS OTHERWISE NOTED.
- PIPE BEDDING SHALL CONFORM TO RCFC&WCD STD. DWG. NO. M815 EXCEPT FOR COVER <2 FEET. FOR COVER <2 FEET. CONCRETE SLURRY (2000 PSI 2 SACK) SHALL BE USED. THE ENTIRE TRENCH SHALL BE SLURRY EXTENDING 4 INCHES MINIMUM AND 12 INCHES MAXIMUM ABOVE THE TOP OF
- BH-1 INDICATES SOIL BORING LOCATIONS BASED ON THE SOILS REPORT DATED MARCH 13, 2001. LOCATIONS SHOWN ARE APPROXIMATE.
- "V" IS THE DEPTH OF CATCH BASINS MEASURED FROM THE TOP OF CURB TO INVERT OF CONNECTOR
- CATCH BASINS SHALL BE LOCATED SO THAT LOCAL DEPRESSION SHALL BEGIN AT EXISTING CURB RETURN JOINT, UNLESS SPECIFIED. 15.
- ALL CURBS, GUTTERS, SIDEWALKS, DRIVEWAYS AND OTHER EXISTING IMPROVEMENTS TO BE RECONSTRUCTED IN KIND AND AT THE SAME ELEVATION AND LOCATION AS THE EXISTING IMPROVEMENTS UNLESS OTHERWISE NOTED.
- STANDARD DRAWINGS CALLED FOR ON THE PLAN AND PROFILE SHALL CONFORM TO DISTRICT STANDARD DRAWINGS UNLESS NOTED OTHERWISE.
- THE CONTRACTOR IS REQUIRED TO CALL ALL UTILITY AGENCIES REGARDING TEMPORARY SHORING AND SUPPORT REQUIREMENTS FOR THE VARIOUS UTILITY LINES SHOWN ON THESE PLANS.
- DURING ROUGH CRADING OPERATIONS AND PRIOR TO CONSTRUCTION OF PERMANENT DRAINAGE STRUCTURES, TEMPORARY DRAINAGE CONTROL SHOULD BE PROVIDED TO PREVENT PONDING WATER AND DAMAGE TO ADJACENT PROPERTIES.
- APPROVAL OF THESE PLANS BY THE RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT DOES NOT RELIEVE THE DEVELOPER'S ENGINEER OF RESPONSIBILITY FOR THE ENGINEERING DESIGN. IF FIELD CHANGES ARE REQUIRED, IT WILL BE THE RESPONSIBILITY OF THE DESIGN ENGINEER TO MAKE THE NECESSARY CORRECTIONS.
- THE CONTRACTOR OR DEVELOPER SHALL SECURE ALL REQUIRED ENCROACHMENT AND/OR STATE AND FEDERAL REGULATORY PERMITS PRIOR TO THE COMMENCEMENT OF ANY WORK.

R.C.F.C. & W.C.D. STANDARD DRAWINGS

CB 110 CONCRETE DROP INLET

MH 252 MANHOLE NO. 2

MH 254 MANHOLE NO 4

M 804 BULKHEAD

COUNTY OF RIVERSIDE STANDARD DRAWINGS

300 CURB INLET BASIN LOCAL DEPRESSION

CONSTRUCTION NOTES

- (1) INSTALL 48" R.C.P. (1000-D MIN.)
- INSTALL 42" R.C.P. (1000-D) MIN.
- INSTALL 24" R.C.P. CLASS IV.
- (4) INSTALL 18" R.C.P. CLASS IX.
- CONSTRUCT MANHOLE NO. 2 PER R.C.F.D. & W.C.D.
- STD. DWG. NO. M.H. 252.

STD. DWG. NO. M804

- (6) CONSTRUCT MANHOLE NO. 4 PER R.C.F.D. & W.C.D. STD. DWG. NO. M.H. 254.
- CONSTRUCT CURB INLET CATCH BASIN PER RIVERSIDE COUNTY STD. DWG. NO. 300.
- (8) CONSTRUCT LOCAL DEPRESSION (CASE C) PER RIVERSIDE
- COUNTY STD. DWG. NO. 311. CONSTRUCT INLET PER R.C.F.C.D. & W.C.D. STD. DWG.
- NO. CB110. CONSTRUCT TIMBER BULKHEAD PER R.C.F.C. & W.C.D.

APPROVED BY DATE_ 2-3-06



998

IP NO. 030086

PROJECT NO.

2-0-0326 DRAWING NO.

2 - 346SHEET NO.

for the location of buried utility lines. Don't disrupt witch services. THO WORKING DAYS BETURE YOU DIO		SEC.	36 1	1 2 S	R 7	w		
CH MARK RSIDE COUNTY B.M. M.L.		REVISIONS	ENGI	NEER	RCFC,	/	DESIGNED BY:	T
1-64 A BRASS DISK SET IN THE							RW DRAWN BY:	+
OF A CONCRETE POST LOCATED							10000	1
HE N.E. COR, OF THE "T" INT.							JM	╛
RANGE ST. AND SUMNER AVE., FT. E. OF SUMNER AVE., 21.0							DATE DRAWN:	T
I. OR ORANGE ST., 2.5 FT. W.				77			DECEMBER 2002	ı
P.P.#375282, 1.0 FT. E. OF A								7
ER" POST. EL.=606.913	REF.	DESCRIPTION	APPR.	DATE	APPR.	DATE		

NORCO DR.

PROJECT LOCATION

MORENO

- VALLEY

PERRIS

EI SINORE

MURRIETA

END

CITRUS ST.

TEMECULA

LAKE

SAN DIÉGO CO.

CLOVERDALE RD.

DRANGE ST

BEGIN

Don't Dig...Until You Call U.S.A. Toll Free

BENC

LINE D-

1-800-227-2600

-24

RIVERSIDE

SAN BERNARDING COUNTY

RESERVOIR

HOT SPRINGS

<u>-</u>

PALM SPRINGS

IMONITE AVE.

68TH ST.

INDEX MAP

6TH ST.

RIVERSIDE COUNTY

PHANNING

SAN JACINTO

VICINITY MAP

SCHLEISMAN RD.

HEMET

LAKE SKINNER

RIVERSIDE CO.

RHA Engineering, Inc. BROCKTON AVENUE • SUIT RIVERSIDE, CA 92506 (909) 683-8088 FAX (909) 683-6915 3-12-04

AND
WATER CONSERVATION DISTRICT RECOMMENDED FOR APPROVAL BY: Shout ETTK.K. DATE: 8-25-2004 DATE: 2/26/09

RIVERSIDE COUNTY FLOOD CONTROL

TRANSPORTATION DEPARTMENT

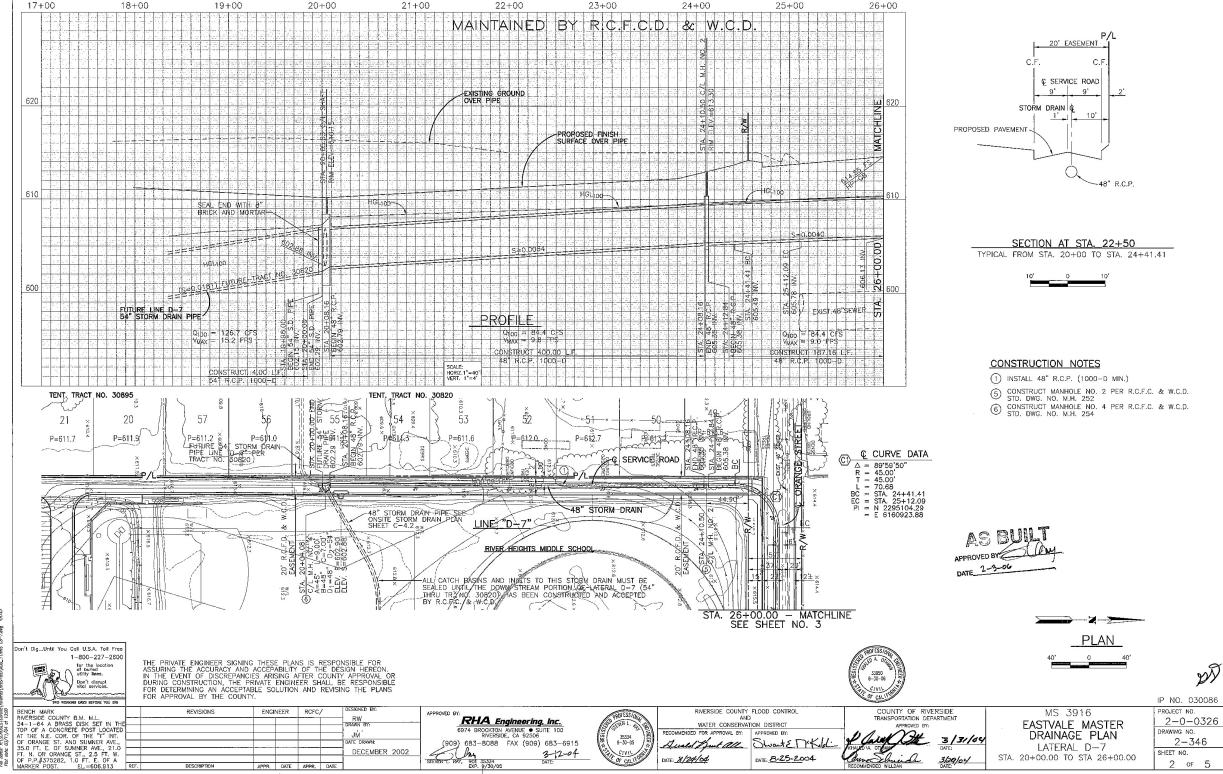
APPROVED BY:

DRAINAGE PLAN LATERAL D-7 TITLE SHEET

MS 3916

EASTVALE MASTER

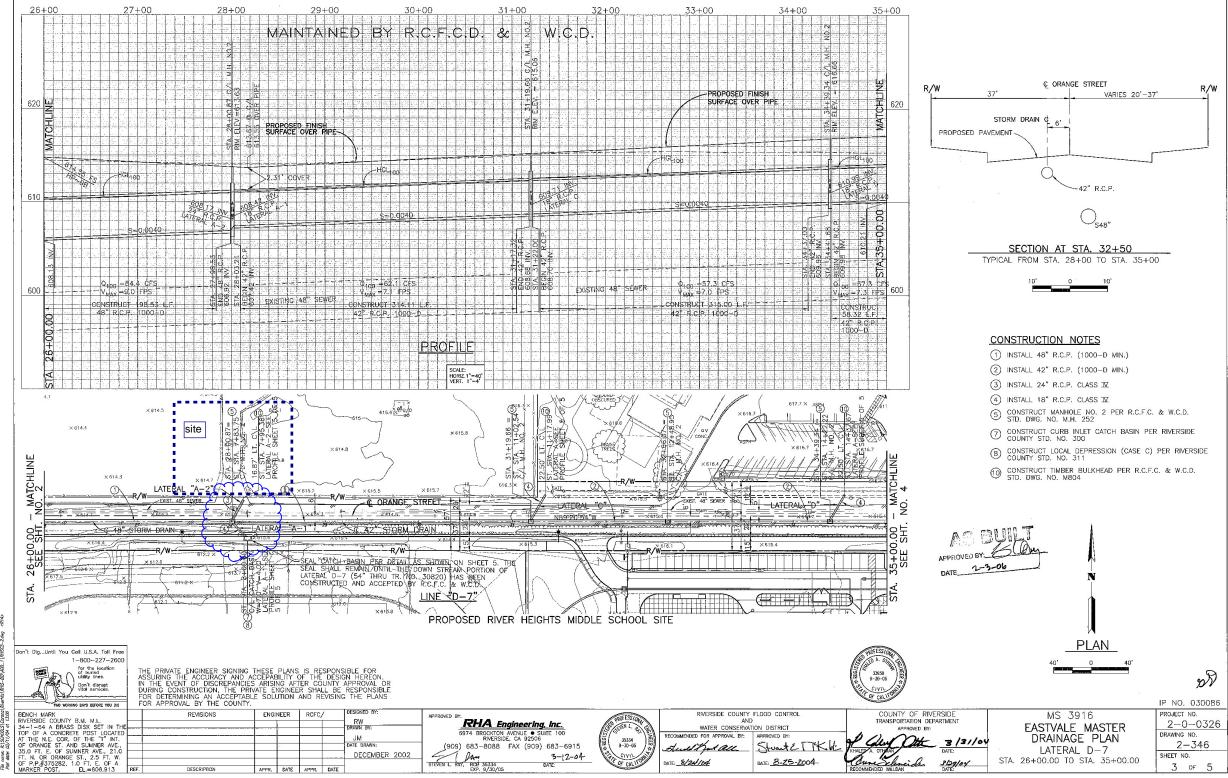
1 or 5

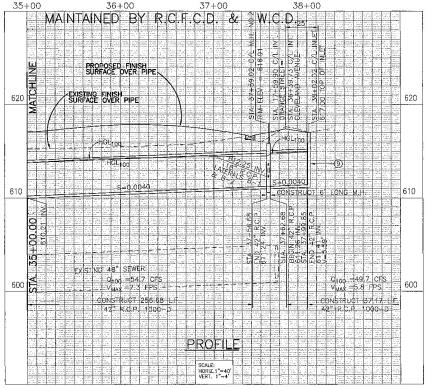


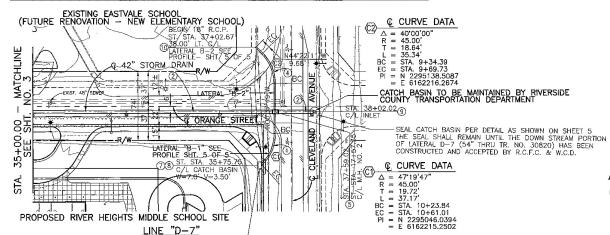
2 or 5

APPR. DATE APPR. DATE

DESCRIPTION







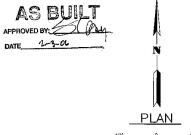
¢ ORANGE STREET R/W STORM DRAIN ¢ PROPOSED PAVEMENT -42" R.C.P S48'

> SECTION AT STA. 36+00 TYPICAL FROM STA. 35+00 TO STA. 37+10



CONSTRUCTION NOTES

- (2) INSTALL 42" R.C.P. (1000-D MIN.)
- (4) INSTALL 18" R.C.P. CLASS IV
- (5) CONSTRUCT MANHOLE NO. 2 PER R.C.F.C. & W.C.D. STD. DWG. NO. M.H. 252
- CONSTRUCT CURB INLET CATCH BASIN PER RIVERSIDE COUNTY STD. NO. 300
- (8) CONSTRUCT LOCAL DEPRESSION (CASE C) PER RIVERSIDE COUNTY STD. NO. 311
- (9) CONSTRUCT INLET PER R.C.F.C. & W.C.D. STD. DWG, NO. CB110
- (10) CONSTRUCT TIMBER BULKHEAD PER R.C.F.C. & W.C.D. STD. DWG, NO. M804



IP NO. 030086 PROJECT NO.

2-0-0326 DRAWING NO. 2-346

Oon't Dig...Until You Call U.S.A. Toll Free 1-800-227-2600 for the location Don't disrupt vital services.

THE PRIVATE ENGINEER SIGNING THESE PLANS IS RESPONSIBLE FOR ASSURING THE ACCURACY AND ACCEPABILITY OF THE DESIGN HEREON. IN THE EVENT OF DISCREPANCIES ARISING AFTER COUNTY APPROVAL OR DURING CONSTRUCTION, THE PRIVATE REGINEER SHALL BE RESPONSIBLE FOR DETERMINING AN ACCEPTABLE SOLUTION AND REVISING THE PLANS FOR APPROVAL BY THE COUNTY.

BENCH MARK
RIVERSIDE COUNTY B.M. M.L.
34-1-64 A BRASS DISK SET IN THE
TOP OF A CONCRETE POST LOCATED
AT THE NE. COR. OF THE "T" INT.
OF ORANGE ST. AND SUMNER AVE., 21.0
FT. N. OR ORANGE ST., 2.5 FT. W.
OF P.P. #375282, 1.0 FT. E. OF A
MARKER POST. EL. ≈606.913 DESIGNED RO ENGINEER RCFC/ REVISIONS DATE DRAWN DECEMBER 2002 DESCRIPTION APPR. DATE APPR.

RHA Engineering, Inc. 6974 BROCKTON AVENUE ● SUITE RIVERSIDE, CA 92506 (909) 683-8088 FAX (909) 683-6915

3-12-04

35334 9-30-05

RIVERSIDE COUNTY FLOOD CONTROL WATER CONSERVATION DISTRICT RECOMMENDED FOR APPROVAL BY: MBM all DATE: 8/24/04

- SEAL CATCH BASIN PER DETAIL AS SHOWN ON SHEET 5 THE SEAL SHALL REMAIN UNTIL THE DOWN STREAM PORTION OF LATERAL D-7 (54" THRU TR. NO. 30820) HAS BEEN CONSTRUCTED AND ACCEPTED BY R.C.F.C. & W.C.D.

APPROVED BY DATE: 8-25-2004

AND

TRANSPORTATION DEPARTMENT APPROVED BY:

COUNTY OF RIVERSIDE

MS 3916 EASTVALE MASTER DRAINAGE PLAN LATERAL D-7 STA 35+00.00 TO STA. 38+02.02

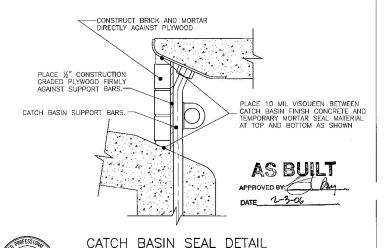
SHEET NO. 4 of 5 10+00 10 + 00ALL CATCH BASING INLETS MUST BE SEALED UNIT THE DOWN STREAM PORTION OF LATERAL D-7 (54) THRU TR. IN 30802) (4S BEEN CONSTRUCTED AND ACCEPTED BY R.C.F.C. & W.C.D. 620 610 CONSTRUCT 73.54 |L.F. Q₁₀₀ =2.2 GFS V_{MAX} =1.2 FPS -GONSTRUCT 18" R.C.P. EXIST 48" SEWER LATERAL "A-1" LATERAL "B-2" LATERAL "B-1" SCALE: HORIZ.1"=40 VERT. 1"=4"

10+00620 610 0100 +4.0 CFS VMAX = 2.3 FPS CONSTRUCT 30 691 LF CONSTRUCT BULKITAD STAL 91-67.72 86-61-30 INV 611-30 INV 611-30 INV 611-30 INV 611-30 INV SCALE: HORIZ.1"=40' VERT. 1"=4'

Q100: 22.8 CFS from the site in proposed condition < 24.7 CFS design capacity of Lat A-2.

PROFILE

LATERALS "A-1", "A-2", "B-1", B-2", "C" & "D" ARE MAINTAINED BY RIVERSIDE COUNTY TRANSPORTATION DEPARTMENT



Don't Dig...Until You Call U.S.A. Toll Free 1-800-227-2600

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- 1	INO NUMBERO DATA DEPONE TOO LIES							A STATE OF THE STA
ı	BENCH MARK		REVISIONS	ENGI	NEER	RCFC,	/	DESIGNED BY:
١	RIVERSIDE COUNTY B.M. M.L. 34-1-64 A BRASS DISK SET IN THE							RW DRAWN BY:
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RHA Engineering, Inc. 6974 BROCKTON AVENUE ● SUITE 100 RIVERSIDE, CA 92506 683-8088 FAX (909) 683-6915 3-12-04



WATER CONSERVATION DISTRICT

RECOMMENDED FOR APPROVAL BY: APPROVED BY Lune Hart All DATE: 8/24/04

APPROVED BY:

COUNTY OF RIVERSIDE TRANSPORTATION DEPARTMENT

MS 3916 EASTVALE MASTER DRAINAGE PLAN

IP NO. 030086 PROJECT NO. 2-0-0326 DRAWING NO. 2 - 346

RIVERSIDE COUNTY FLOOD CONTROL

DATE: 8-25-2004

LATERAL D-7 COLLECTOR PIPES

SHEET NO. 5 of 5